



Seth Bradley, PE Senior Principal Engineer



Evaluating River-Influenced Groundwater Infiltration and Sewer Rehabilitation Effectiveness



Agenda

- Background
- Flow Monitoring Analysis
- Representing River-Influenced Infiltration
- Model Calibration
- Capacity Analysis
- Sewer Rehabilitation Effectiveness
- Conclusions

Miami Shores LS Study Area

Montgomery County, OH

- 575 acres
- 10 miles gravity sewer
- 1,400 customers
- 0.90 MGD ADF



Miami Shores LS

- Two submersible pumps
- ~2,700' of 10" force main
- FM mostly cast iron
- FM crosses Great Miami River suspended from Main Street bridge (~700' steel pipe)
- Firm Capacity ~ 2.4 MGD

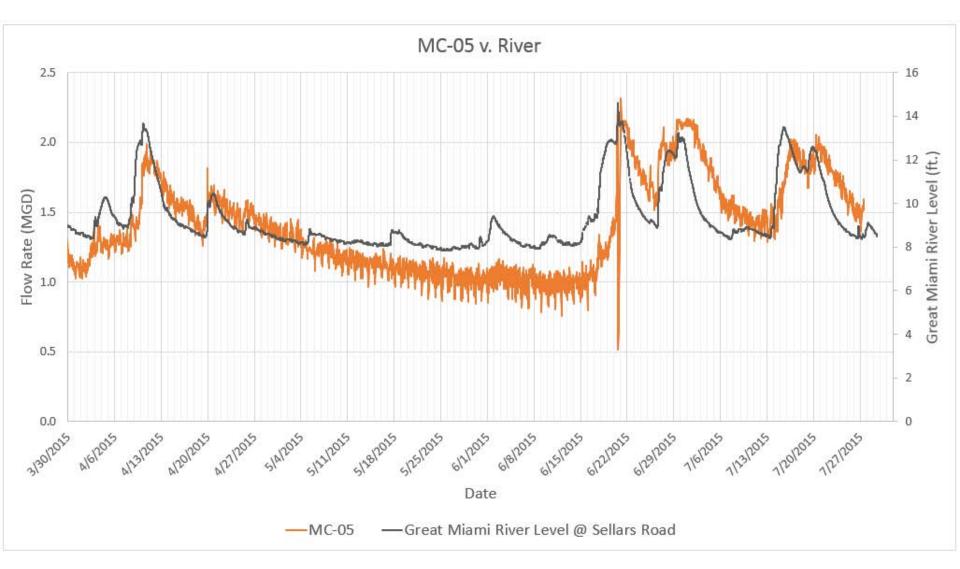


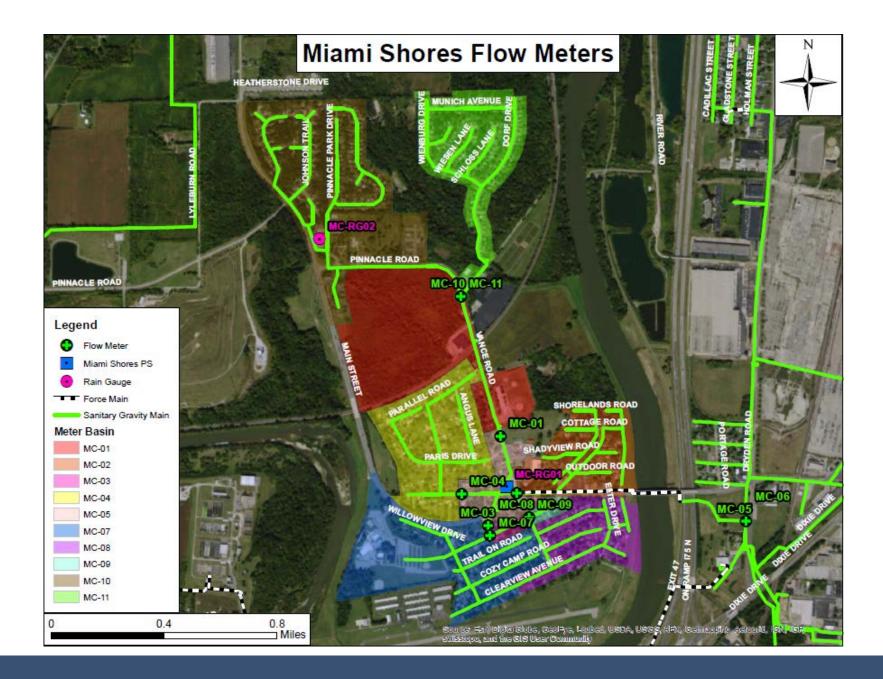
Miami Shores LS

- Significant I/I upstream due to high groundwater exceeding LS capacity.
- Only 1 pump operated at a time due to electrical and force main capacity limitations
- Some sewers upstream/downstream of LS undersized
- Sewer and manhole rehab work previously done upstream of LS

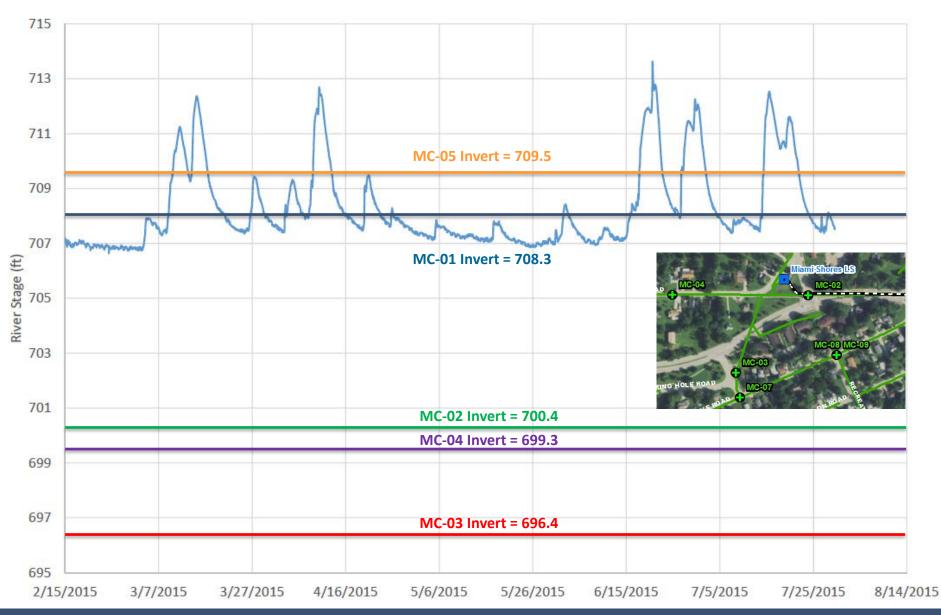


Miami Shores LS Flow Rate

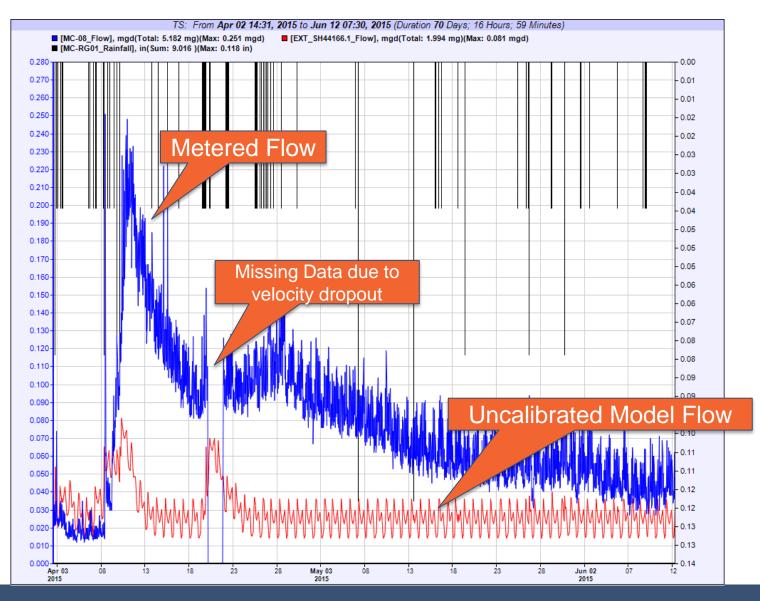




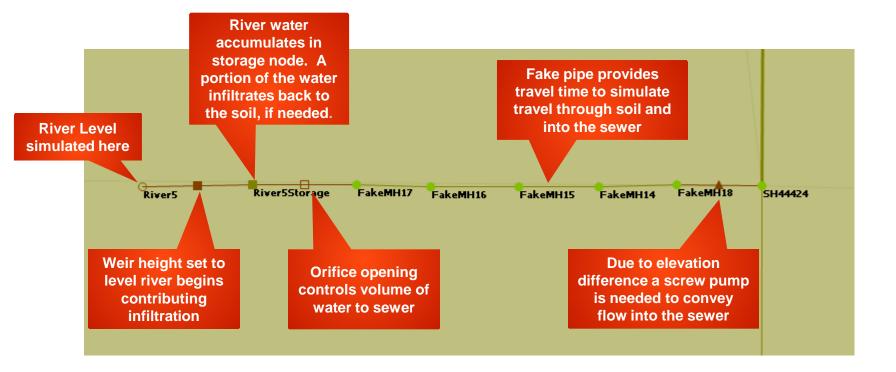
USGS 03271207 – Great Miami River at Sellars Rd.



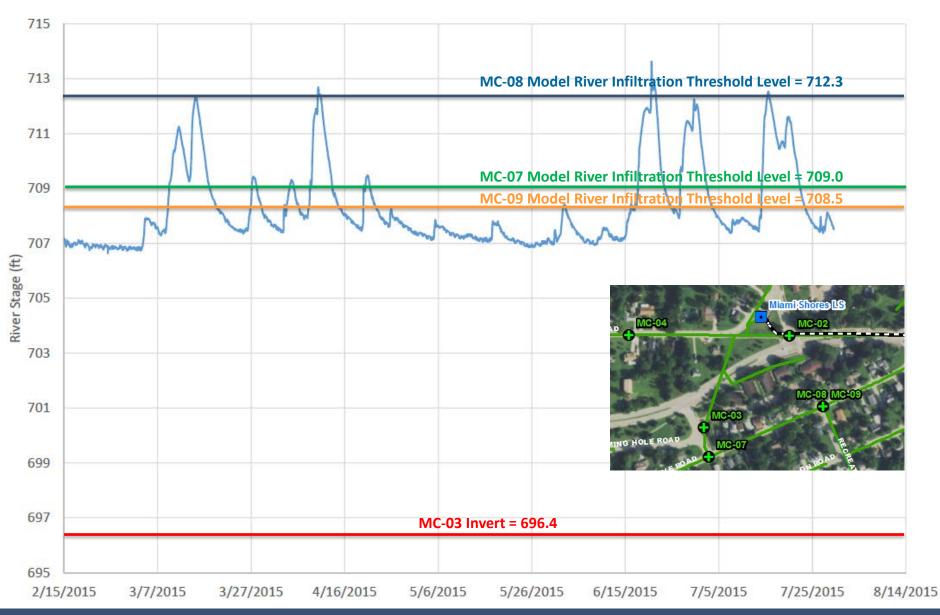
River-Influenced Infiltration – MC-08



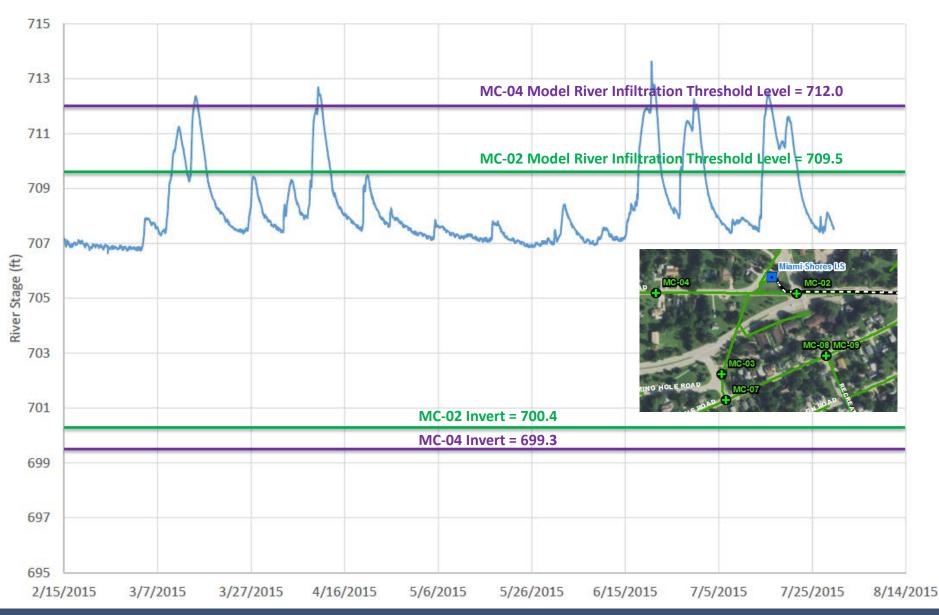
Model Representation of River Influence



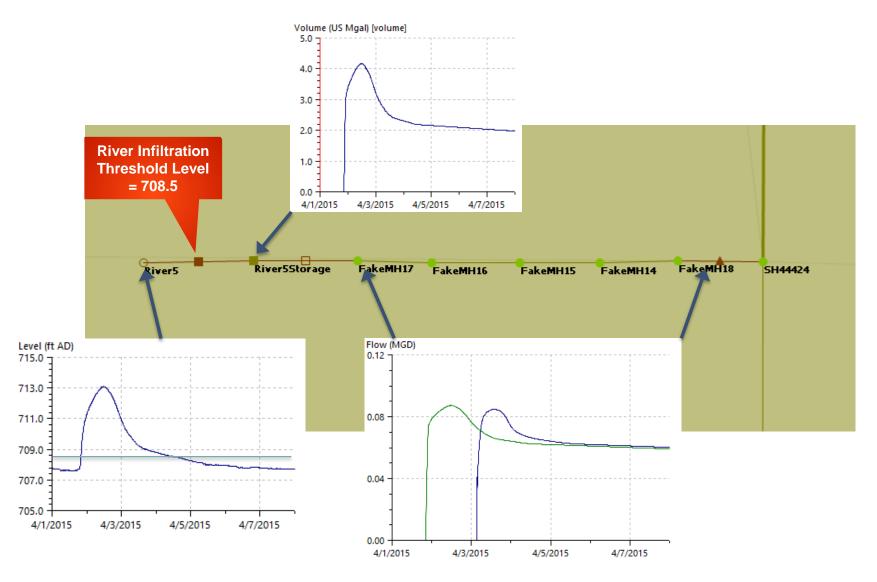
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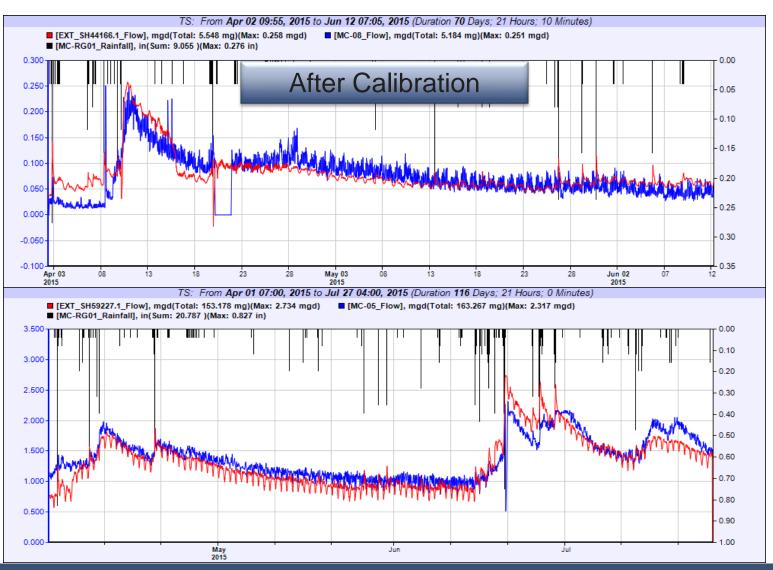
Model Representation of River Influence – MC-09

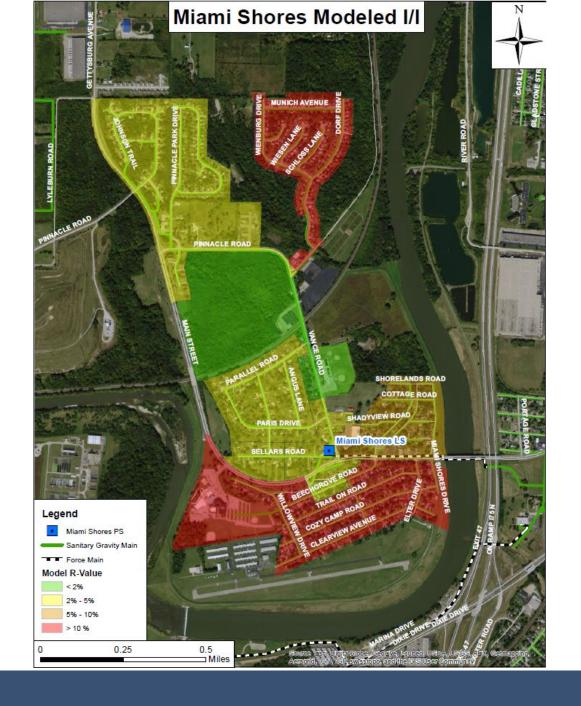


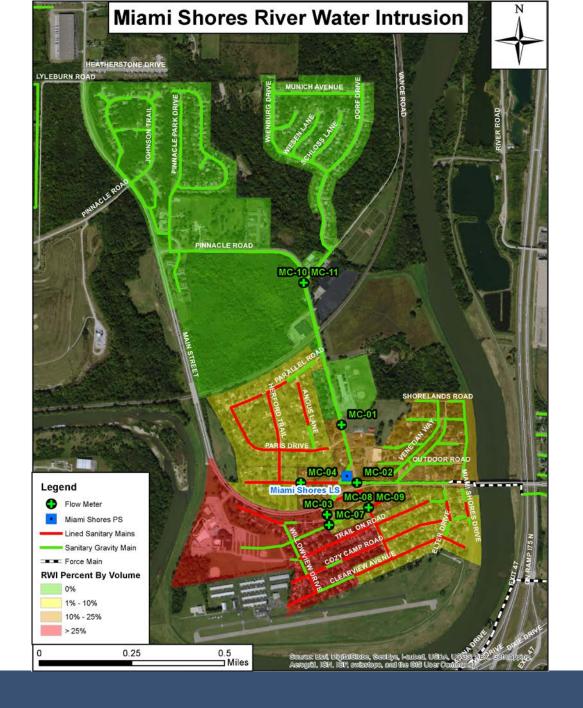
Wet Weather Model Calibration Results

Flow Meter	Model Volume (MG)	Meter Volume (MG)	% Difference	Model Peak Flow (MGD)	Meter Peak Flow (MGD)	% Difference	Model Level (ft.)	Meter Level (ft.)	Difference (ft.)				
April 10 – May 25, 2015 (high river levels)													
MC-01	14.76	11.43	+29%	0.62	0.79	-22%	0.47	0.88	-0.41				
MC-02	3.41	3.81	-10%	0.226	0.225	0%	0.34	0.39	-0.05				
MC-03	Bad Meter Data												
MC-04	1.03	0.91	+13%	0.08	0.09	-11%	0.18	0.25	-0.07				
MC-05	51.44	60.35	-15%	1.90	2.22	-14%	0.80	0.79	0.01				
MC-07	21.59	18.91	+14%	0.73	0.70	+4%	5.09	5.09	0.01				
MC-08	3.91	4.05	-3%	0.26	0.25	+4%	3.99	3.87	0.11				
MC-09	4.70	4.09	+15%	0.16	0.37	-57%	1.98	1.81	0.17				
June 19 – June	22, 2015 (nearly	2YR6HR storm)											
MC-01	1.69	1.48	+14%	1.22	1.00	+22%	1.32	0.84	0.48				
MC-02	0.51	0.48	+6%	0.35	0.27	+30%	0.69	0.40	0.29				
MC-03	3.27	2.83	+16%	1.37	1.44	-5%	8.90	6.17	2.72				
MC-04	0.10	0.07	+43%	0.08	0.10	-20%	1.82	0.22	1.60				
MC-05	5.49	4.92	+12%	2.42	2.60	-7%	8.42	10.71	-2.29				
MC-10	0.86	0.74	+16%	1.29	1.25	+3%	0.33	0.46	-0.14				
MC-11	0.79	0.66	+20%	0.63	0.47	+34%	1.04	0.91	0.13				
June 28 – July 2	26, 2015 (high rive	er levels)											
MC-01	10.77	8.51	+27%	1.05	0.71	+48%	0.73	0.55	0.18				
MC-02	4.39	4.44	-1%	0.26	0.30	-13%	0.35	0.44	-0.09				
MC-03	25.90	25.46	+2%	1.18	1.26	-6%	5.91	6.94	-1.04				
MC-04	1.34	1.40	-4%	0.09	0.12	-25%	0.18	0.22	-0.03				
MC-05	42.79	48.86	-12%	2.40	2.30	+4%	0.80	0.80	0.00				
MC-10	6.39	6.72	-5%	0.76	0.47	+62%	0.24	0.25	-0.01				
MC-11	4.00	4.05	-1%	0.37	0.51	-27%	0.59	0.51	0.08				

MC-05 & MC-08 Calibration Results

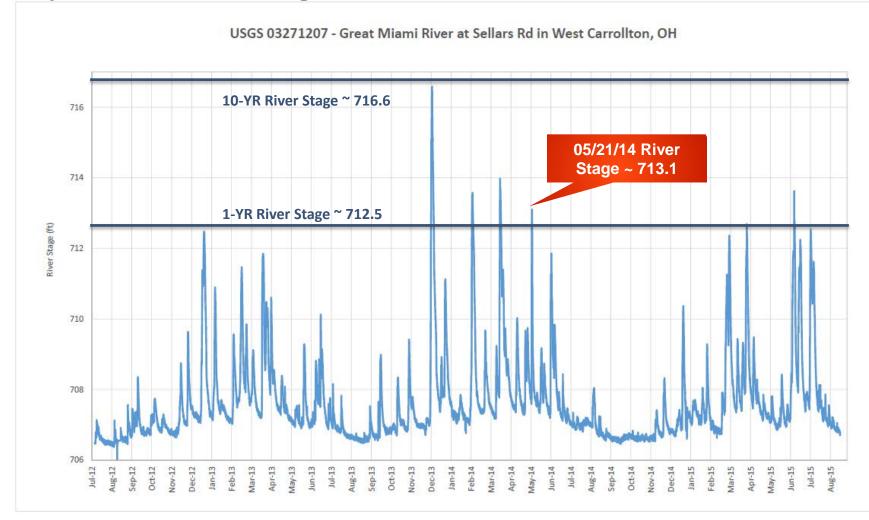




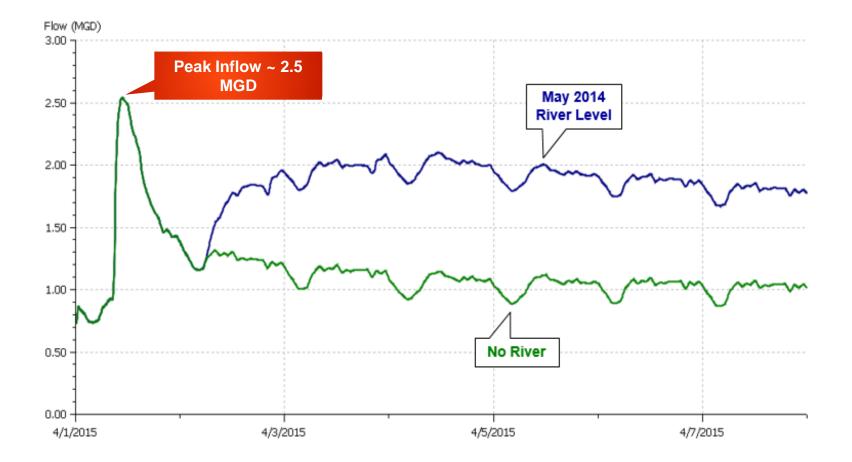


Capacity Analysis River Level

May 21, 2014 ~ 5YR River Stage



Miami Shores 5YR6HR Influent



Capacity Limited Areas – 5YR6HR

729.0

727.0 --725.0 --723.0 --721.0 --711.0 --711.0 --713.0 --713.0 --711.0 --709.0 --707.0 --

ft

Link

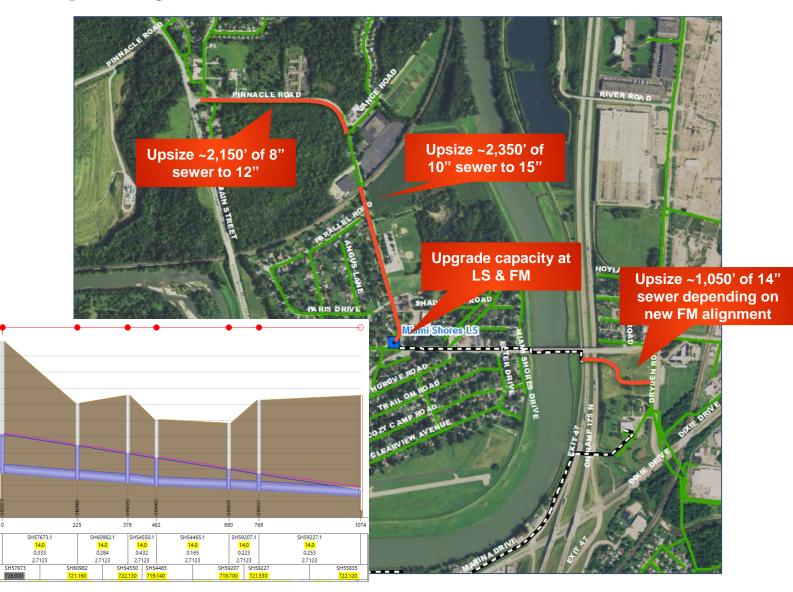
width (in)

DS flow (MGD)

ground (ft AD)

grad (%)

Node



Miami Shores LS Peak Inflows

5YR6HR Design Storm

Scenario	Peak Flow (MGD)
Existing Pipes, Existing Flows	2.54
Upsized Vance Rd, Existing Flows	2.75
Existing Pipes, Future Flows	2.82
Upsized Vance Rd, Future Flows	3.03

Notes:

- 1) DARF = 0.94 (same as Master Plan), antecedent rainfall = 3.5"
- 2) GWI SSD initially 5% below threshold (same as Master Plan)
- 3) Model simulation used outfall upstream of Miami Shores LS
- 4) Assuming sewer blockage upstream of Miami Shores LS is removed
- 5) Existing pipes used (not open system)
- 6) May 2014 River conditions
- 7) Required Vance Rd sewer upsizing is 12" for existing flows, 15" for future flows.

Capacity Evaluation Conclusions

- Unique approaches are required to model riverinfluenced infiltration.
- For this case, river water infiltration did not significantly impact system capacity limitations.

Sewer Rehabilitation Effectiveness

Prior Rehabilitation Efforts

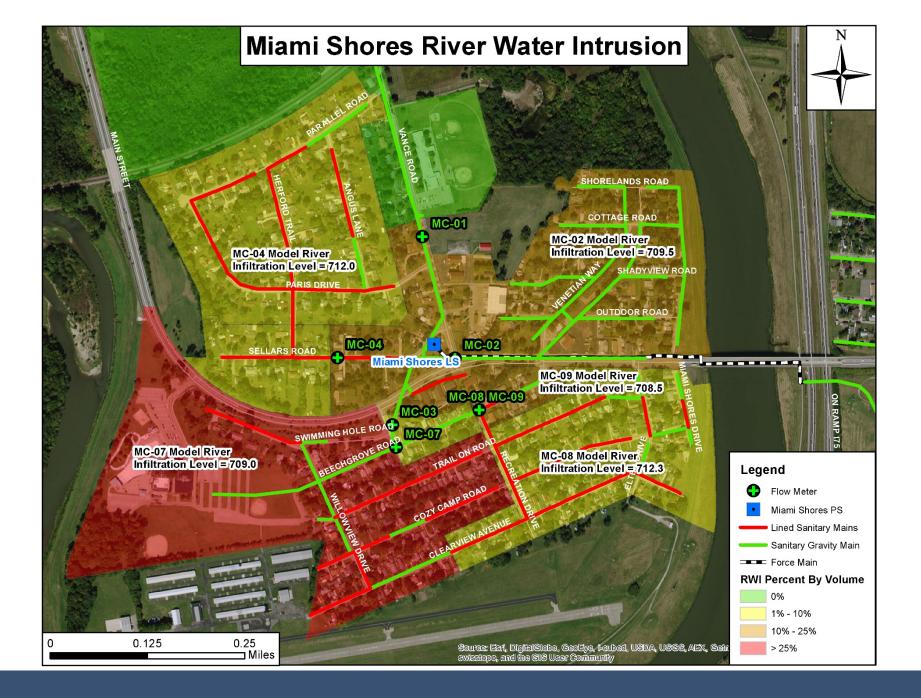
- 14,600' of gravity sewer;
 27 manholes upstream of Miami Shores LS.
- Full length CIPP with seals at each end
- Manholes lined with cementitious liner



Impact of Prior Rehabilitation Efforts

(14,600 ft. of sewer lined and 27 MHs rehabilitated)

- No flow data prior to rehab
- Pump run times not usable due to pump replacements and bypass pumping
- Prior rehabilitation appears effective at reducing RWI where >75% sewer length lined
- I/I (R-values and RWI) still high, even in most basins with < 75% lined sewers
- Other than grouting first 3', laterals were not rehabilitated, which reduced effectiveness





Develop Comprehensive Rehabilitation Costs

Develop Meter Basin Statistics:

Meter	Total Gravity	Percent Unlined	Number of	Non-Rehabed	Number of	Length of Laterals (ft)		
Basin	Sewer (ft)	Sewers (%)	Manholes	Manholes (%)	Services			
MC-02	7,959	100%	39	90%	86	4,251		

Determine Additional Rehabilitation Methods:

Meter Basin	Sewer Lining	Manhole Rehab	Full Lateral Rehab	ral Lateral Add		Remove Direct Connections	Anticipated Non River I/I Reduction (%)	Anticipated RWI Reduction (%)	
MC-02	Х	Х	Х		Х	Х	50%	50%	

Calculate Rehabilitation Quantities:

Meter Basin		10" Sewer Lining (ft)	MH Chimney Seals & Solid Lids	MH Rebab (Vertical ft)	Lateral Rehab & Cleanout (Qty)	Lateral Rehab (ft)	Remove Direct Connections	
MC-02	6,222	1,737	35	243	86	4,251	9	

Comprehensive Rehabilitation Costs

	Testin	g Cost		Rehabilitation Cost																
								M	I Chimney										TOTAL Testing &	
Meter	Smoke	Dye	8" Se	wer	10" Sewer	12" Sewe	r 15" Sewer	Sea	als & Solid			Lateral		Add	Rem	nove Direct	Co	ntingency	Re	habilitation
Basin	Testing	Testing	Linir	ing	Lining	Lining	Lining		Lids	MH Rehab		Rehab	Cleanouts		Connections		(30%)		Costs	
MC-01	\$-	\$-	\$	-	\$-	\$-	\$ -	\$	-	\$-	\$	-	\$	-	\$	-	\$	-	\$	-
MC-02	\$ 3,980	\$-	\$ 279	9,999	\$ 86,850	\$-	\$-	\$	17,500	\$ 30,431	\$	419,555	\$	172,000	\$	31,500	\$	311,350	\$	1,353,165
MC-03	\$-	\$-	\$	-	\$ 38,500	\$-	\$-	\$	-	\$-	\$	65,577	\$	34,000	\$	-	\$	41,423	\$	179,500
MC-04	\$-	\$-	\$	-	\$-	\$-	\$-	\$	-	\$-	\$	-	\$	-	\$	-	\$	-	\$	-
MC-05	\$-	\$-	\$ 27	7,180	\$ 69,550	\$-	\$-	\$	-	\$-	\$	111,157	\$	54,000	\$	-	\$	78,566	\$	340,453
MC-07	\$ 3,430	\$ 46,200	\$ 99	9,990	\$ 54,350	\$-	\$-	\$	10,000	\$ 20,211	\$	663,955	\$	264,000	\$	24,500	\$	341,102	\$	1,527,738
MC-08	\$ 2,786	\$ 40,950	\$ 52	2,785	\$-	\$-	\$ -	\$	12,500	\$ 28,369	\$	593,242	\$	234,000	\$	21,000	\$	282,569	\$	1,268,200
MC-09	\$ 406	\$ 4,550	\$ 10	0,845	\$-	\$-	\$ -	\$	1,500	\$ 3,433	\$	61,532	\$	26,000	\$	3,500	\$	32,043	\$	143,809
MC-10	\$ 6,388	\$ 62,650	\$ 326	6,655	\$ -	\$-	\$ -	\$	-	\$-	\$	721,598	\$	358,000	\$	63,000	\$	440,776	\$	1,979,067
MC-11	\$ 4,138	\$ 67,900	\$ 259	9,898	\$-	\$ 2,600	\$ 40,280	\$	-	\$-	\$	766,026	\$	388,000	\$	136,500	\$	477,991	\$	2,143,332
TOTAL	\$21,126	\$222,250	\$1,057	7,352	\$249,250	\$ 2,600	\$ 40,280	\$	41,500	\$ 82,444	\$3	3,402,641	\$:	1,530,000	\$	280,000	\$	2,005,820	\$	8,935,263

Cost Savings From Potential Rehabilitation

		Annu	ial C	0&M Sav	/ing	,s	Capital Improvemen		20-Year	
Meter	IV	Miami	i Dryden		W	/estern	Vance Rd. Sewer	Miami Shores FM	Pre	sent Worth
Basin	Shc	ores LS	S Rd PTF		Re	egional	Replacement	Upsize	1	Savings
MC-02	\$	320	\$	120	\$	880			\$	21,600
MC-03	\$	50	\$	20	\$	140			\$	3,400
MC-05	\$	560	\$	220	\$	1,580			\$	38,700
MC-07	\$	1,620	\$	620	\$	4,530			\$	111,000
MC-08	\$	460	\$	180	\$	1,280			\$	31,500
MC-09	\$	610	\$	230	\$	1,720			\$	42,000
MC-10	\$	170	\$	70	\$	470			\$	11,600
MC-11	\$	240	\$	90	\$	670			\$	16,400
Total System	\$	4,030	\$	1,550	\$	11,270	\$ 754,650	\$ 31,200	\$	1,062,100
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Rehabilitation Conclusions

- Prior CIPP Lining was somewhat effective at reducing I/I in areas where nearly all sewers were lined.
- Comprehensive rehabilitation does not appear cost effective for purposes of I/I reduction.
- Rehabilitation should still be considered to extend remaining useful life of assets.



Seth Bradley, PE

859-219-1126 sbradley@hazenandsawyer.com

